

A22.9

$$F = 1500 \text{ N}$$

$$\therefore F_{\text{ut}} = 1250 \text{ N}$$

$$D_0 \leq 0.96 \times 70 = 67 \text{ mm.}$$

$$F = 1000 \text{ N}$$

$$F_s = 250 \text{ N.}$$

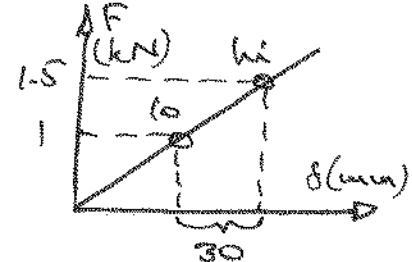
Design via the Goodman approach of (5b) in which

$$F_c = 2F_{\text{ut}} CK_s / (\sigma_{\text{ut}}/\sigma_{\text{ut}})$$

$$+ 2F_s CK_h / (\sigma_{\text{ut}}/\sigma_{\text{ut}})$$

$$= 2 \times 125 CK_s / 0.63 + 2 \times 0.25 CK_h / 0.13 \text{ kN}$$

$$= C (3570 \text{ Ks} + 3850 \text{ Kh}) \text{ N.}$$



*

Assuming a design factor of 1.1

Adopting an approach similar to worked ex.
Twice $C = 7.5$ then

$$F_c = 7.5 (3570 (1 + \frac{1}{15}) + 3850 \frac{2.5+0.6}{2.5-0.9}) = 66.0 \text{ kN}$$

$$\therefore F_{\text{ut}} \geq 1.1 \times 66 = 72.6 \text{ kN} \quad (\text{eq 5b})$$

Try $d = 8$ with $F_{\text{ut}} = 62.8$ (table).

$$\therefore F_{\text{necessary}} = 62.8 / 1.1 = 57.1 \text{ kN}$$

so solve * above for C by iterating

$$C = \frac{57.1}{3570 \text{ Ks} + 3850 \text{ Kh}}$$

$$C : 7.5 \quad 6.49 \quad 6.37 \quad 6.35 \quad \boxed{6.35}$$

$$\text{For } C = 6.35 \quad D = 6.35 \times 8 = 50.8 \text{ mm}$$

$$\therefore D_0 = D + d = 50.8 + 8 = 58.8 \leq 67 \text{ mm - OK.}$$

Now, having apparently settled major parameters, look at constraints.

$$K = (1.5 - 1)/30 = 16.7 \text{ N/mm} \quad I = \frac{\pi d^4}{8 \times 2 \times C^3}$$

$$\therefore u_2 = 7.9 \times 3 \times 8 / 8 \times 16.7 \times 6.35^3$$

= 18.5 pretty large; buckling prob?

$$n_t = 20.5 \quad \text{Table 1 - severed & ground.}$$

$$L_s = 20.5 \times 8 = 164 \text{ mm.}$$

$$\text{Now } \delta_{\text{bi}} = E_{\text{bi}}/k = 1500/16.7 = 90 \text{ mm}$$

Assume 10% elastic deflection, so

$$\delta_s \geq 1.1 \delta_{\text{bi}} = 99 \text{ mm.}$$

$$\therefore L_0 = L_s + \delta_s = 164 + 99 = 263 \text{ mm.}$$

For buckling safety from (3)

$$\begin{aligned} L_{\text{crit}} &= \frac{1}{2} \sqrt{E_i \delta_{\text{bi}}} \left[1 + \left(\frac{c_2 D}{c_1 \gamma \delta_{\text{bi}}} \right)^2 \right] \\ &= \frac{1}{2} \times 1.23 \times 90 \left[1 + \left(2.62 \times 50.8 / (1.23 \times 0.5 \times 90) \right)^2 \right] \\ &= 325 \text{ mm - no end rotation.} \end{aligned}$$

Since $L_0 \leq L_{\text{crit}}$, buckling is no problem.

Check yield when solid - $S_y = 0.48 \times S_{\text{ut}}$

$$\delta \text{ from Table 2} \quad S_y = 0.48 \frac{263 \times 8 / (278 + 56 \times 8)}{1 + 8 (1.6 + 0.48 \times 8)}$$

$$= 605 \text{ MPa}$$

$$\delta \tau_s \text{ from (1)} = 1.08 \times 8 (16.7 \times 90) \times 6.35 / \pi \times 8^2 = 455 \text{ MPa}$$

$\delta \tau_s < S_y$ so OK.