

$$1. \quad 2cz = r^2 \therefore cz' = r ; cz'' = 1 ; \frac{1}{r} = \frac{d}{dr}$$

Equation of normal through (r_1, z)

$$z' = r/(z_0 - z) \text{ i.e. } z_0 = z + r/z'$$

\therefore Length of normal, r_θ , is given by

$$r_\theta^2 = (z_0 - z)^2 + r^2 = r^2(1 + 1/(z')^2) = r^2 + c^2$$

$$\& r_\theta = (r^2 + c^2)^{1/2}$$

The radius of curvature, r_{cf} , is

$$r_{\text{cf}} = [1 + (z')^2]^{3/2}/z'' \text{ and from above:}$$

$$= [1 + (r/c)^2]^{3/2} c = (r^2 + c^2)^{3/2}/c^2$$

Substitute into membrane stress equations (2):-

$$\sigma_{\text{pp}} = (\rho/2t)r_\theta = k(r^2 + c^2)^{1/2} ; k = \rho/2t$$

$$\sigma_\theta = \sigma_{\text{pp}}[2 - r_\theta/r_{\text{cf}}] = k(r^2 + c^2)^{1/2}[2 - c^2/(r^2 + c^2)]$$

$$= k(2r^2 + c^2)/(r^2 + c^2)^{1/2} \quad \text{QED.}$$

For the design of the glass dome, fairly obviously the membrane stresses are maximum when r is maximum. Inserting values, assuming pressure and stresses in MPa and all dimensions in mm

$$\sigma_{\text{pp}} = k(150^2 + 60^2)^{1/2} = 161.6 \text{ k} \quad ? \text{ so the hoop stress}$$

$$\sigma_\theta = k(2 \times 150^2 + 60^2)/161.6 = 300.8 \text{ k} \quad \sigma_\theta \text{ is greater}$$

Setting $\sigma_\theta = 35 \text{ MPa}$, the design stress

$$\rho = \rho g h = 10(\text{kn/m}^3) \times 200(\text{m}) = 2 \text{ MPa}$$

$$\text{Then } 35 = 300.8 \rho/2t ; t = 300.8 \times 2 / 2 \times 35 = 8.6 \text{ mm}$$

2(b) σ_{pp} may be found from free body sketched, then σ_θ follows from membrane equation.

For the free body, $p = \rho(h-z)$

$$\therefore \sigma_{\text{pp}} \cos \alpha \cdot t \cdot 2\pi r = pA + W = pA + \rho \frac{1}{3} A z \text{ (rest of cone)}$$

where $A = \pi r^2$ and $r = z \tan \alpha$.

$$\therefore 2\pi r t \cos \alpha \cdot \sigma_{\text{pp}} = A[\rho(h-z) + \frac{1}{3}\rho z] = \pi r^2 \rho (3h - 2z)/6$$

$$\therefore \sigma_{\text{pp}} = k(3h - 2z)z ; k = \rho \tan \alpha / 6 + \cos \alpha$$

Now require the radius r_θ , r_{cf} for membrane equation.

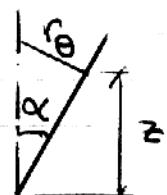
The radius of curvature of the meridian $\rightarrow \infty$

i.e. $r_{\text{cf}} \rightarrow \infty$. From the sketch, $r_\theta = z \tan \alpha$ since

so the membrane equation (ii) becomes:-

$$\sigma_\theta = r_\theta [\rho/t - \sigma_{\text{pp}}/r_{\text{cf}}] = (\rho/t)r_\theta ; \rho = \rho(h-z)$$

$$= \rho(h-z) \frac{1}{t} \cdot z \sin \alpha / \cos^2 \alpha = k(6z(h-z))$$



(b)

Putting the membrane equations & $\sigma_3 = 0$ into dist² equation

$$2\sigma_E^2 = (\sigma_\theta - 0)^2 + (\sigma_{\text{pp}} - 0)^2 + (\sigma_\theta - \sigma_{\text{pp}})^2$$

$$2(\sigma_E/kz)^2 = 36(h-z)^2 + (3h-2z)^2 + [6h-6z-3h+2z]^2$$

$$\sigma_E^2 = (kz)^2 [27h^2 - 54hz + 28z^2]$$

Differentiating for max σ_E :-

$$\frac{d(\sigma_E^2)}{dz} = 0 = 56z^2 - 81hz + 27h^2 ; h = 6.5 \text{ m}$$

$$\therefore z = 6.015 \text{ m or } 3.386 \text{ m}$$

2 (concl'd) It may easily be shown that the latter smaller value of \bar{z} corresponds to $\max \sigma_E$, i.e. σ^* . So

$$\sigma^*/k = 3.386 [27 + 6.5^2 - 54 + 6.5 \times 3.386 + 28 \times 3.386^2]^{1/2} = 56.0 \text{ m}^2$$

& $= \sigma^* b t \cos \alpha / \rho \tan \alpha ; \rho = 9.81 \text{ kN/m}^3$

$$\therefore L = \frac{56.0 \times 9.81 \times 10^3 \times 1}{6 \times 6.5 \times 10^6 \times 1/\sqrt{2}} = 1.99 \text{ mm}$$

$\frac{\text{m}^2}{\text{m}^3}$ $\frac{\text{N}}{\text{m}^2}$ $\text{deg } 2 \frac{\text{mm}}{\text{m}}$

At water level, $z = h$, so membrane stresses from above

$$\sigma_\theta = 0 ; \sigma_\phi = k \cdot h^2$$

$$\therefore \epsilon_\theta : \epsilon_\phi = (\sigma_\theta - \nu \sigma_\phi)/E = -\nu \rho \cdot h^2 \tan \alpha / b E t \cos \alpha$$

$$= -0.3 \times 9.81 \times 10^3 \times 6.5^2 \times 1 / 6 + 207 \times 10^9 \times 2 \times 10^{-3} \times \frac{1}{\sqrt{2}}$$

$$= -71 \times 10^{-6} \text{ i.e. expression of } \gamma \text{ in strain.}$$